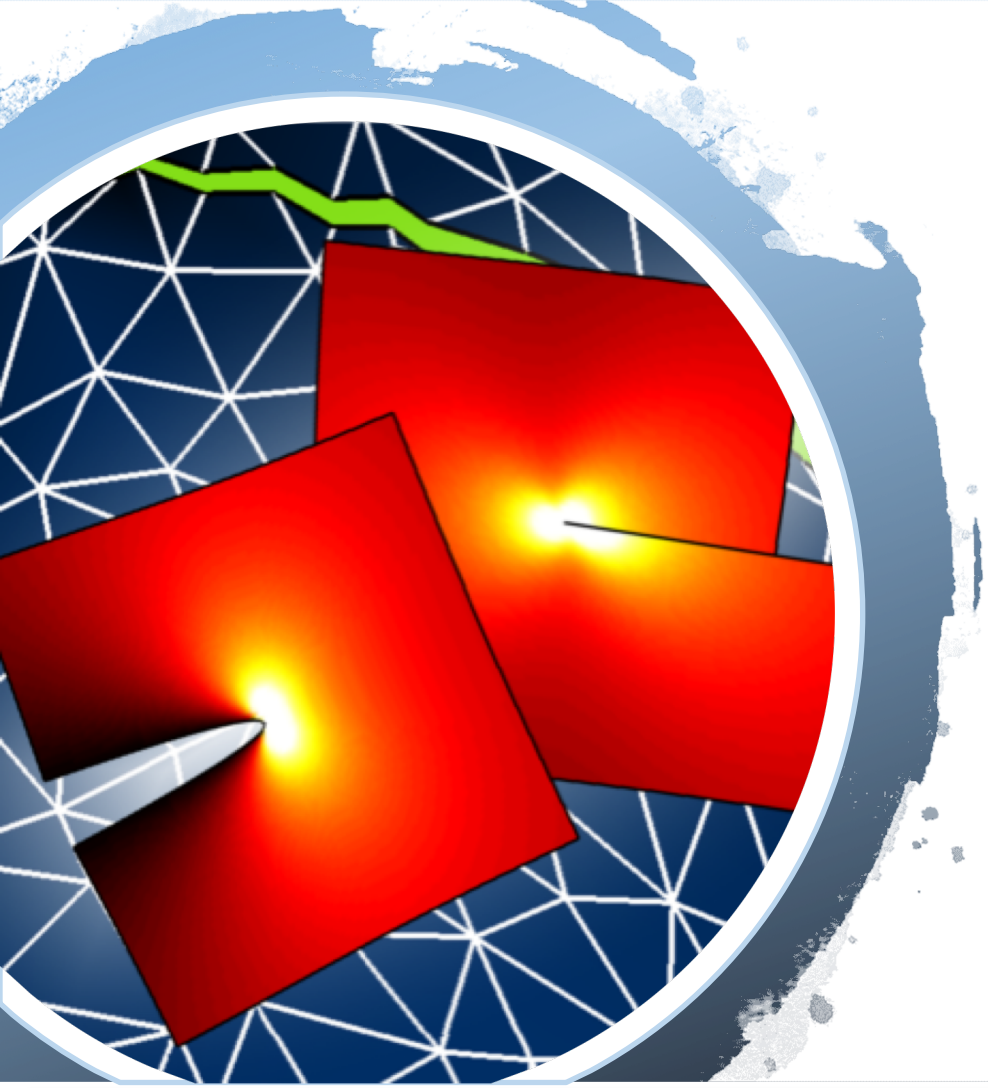


# Extended Finite Element Method

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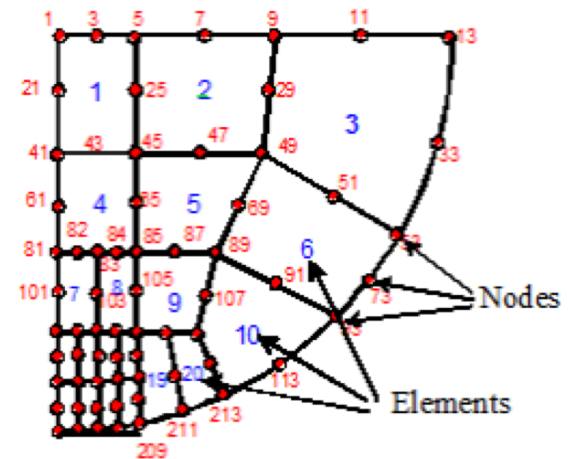
# Introduction



# Introduction

## ➤ Finite Element Method (FEM)

- A numerical tool to obtain approximate solutions of PDEs.
- Steps:
  - Discretization of the solution region;
  - Derivation of equations;
  - Assemble all the elements;
  - Solving equations.
- Requirements:
  - the mesh has to conform to the geometry;
  - Remeshing at each step.



Mesh in FEM

Allan F. Bower, "Applied Mechanics of Solids".

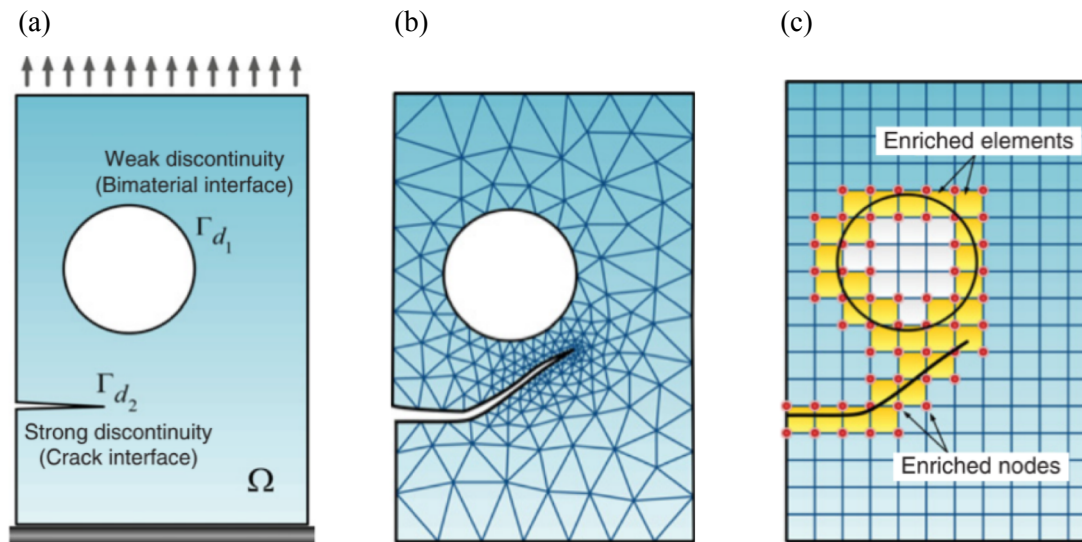
# Introduction

## ➤ **Extended Finite Element Method (XFEM)**

- A powerful tool for discontinuous problems.
- Enables accurate capture of non-smooth features.
- Avoid using a mesh that conforms to cracks with overcoming remeshing difficulties as in FEM.
- Enrich the elements near the crack tip and along the crack faces.

# Introduction

## ➤ FEM vs. XFEM Performance



**Modeling of discontinuities in FEM and XFEM. (a) Crack propagation in a plate with a hole; (b) FEM using adaptive mesh refinement; (c) XFEM with enrichment of the elements.**

*Extended Finite Element Method: Theory and Applications, First Edition. Amir R. Khoei.  
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# Introduction

## ➤ Comparisons

- XFEM vs. FEM
  - XFEM : useful for discontinuities problems since enrichment functions are added to FEM; cracks can propagate along a natural arbitrary path.
  - FEM: poor for arbitrary discontinuities problems; cracks only propagate along the element edge.
- Boundary Element Method (BEM) vs. XFEM
  - It is applicable for multi-material/ phase problems.

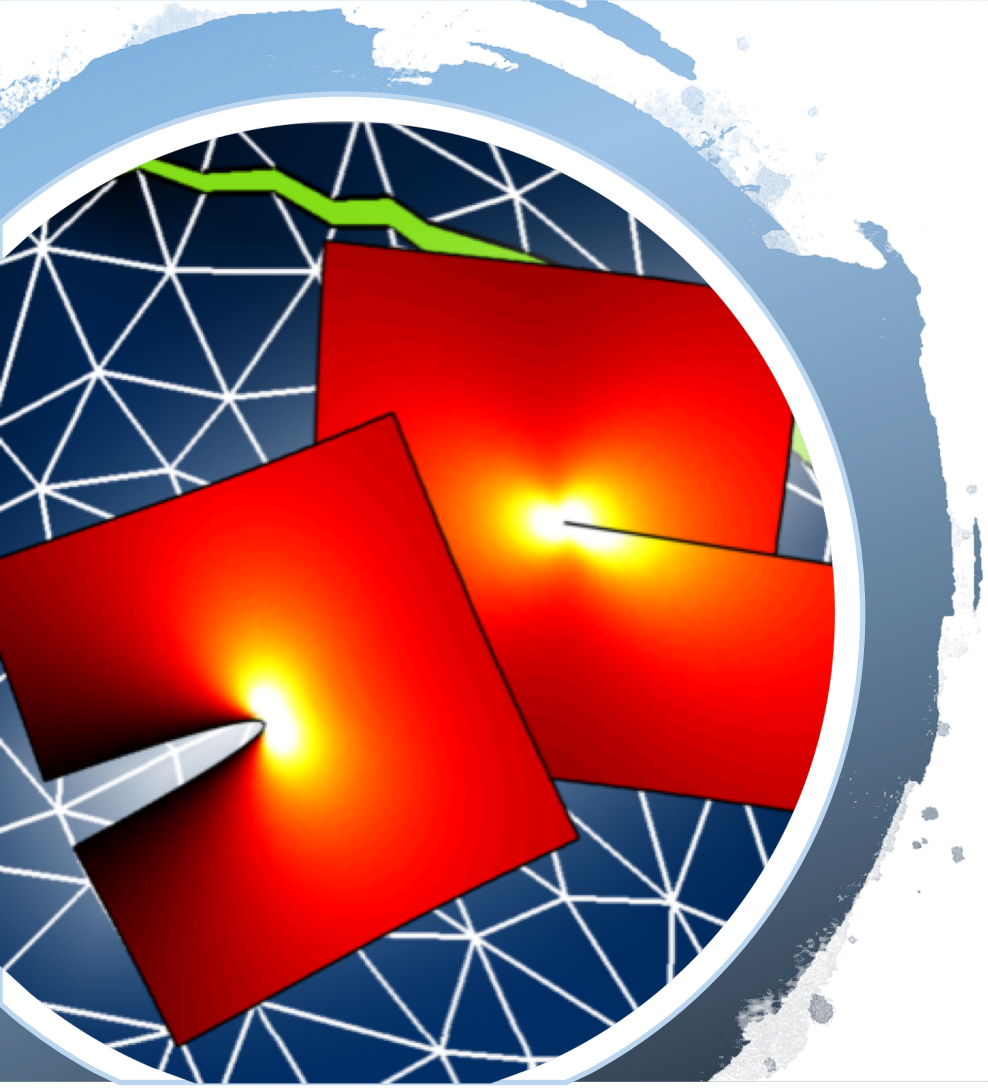
# Introduction

## ➤ **Advantages of XFEM**

- Cracks with complex geometry can be modeled
- No need of remeshing
- Less expensive

## ➤ **Shortcomings of XFEM**

- Hard to localize the initial fracture
- Only for linear elastic fracture mechanics



# Historical Perspective

# Historical Perspective

## ➤ **Ted Belytschko and Tom Black (1999)**

- A improved technique for finite elements based on a partition-of-unity which involves minimal remeshing.
- Not yet applicable for long cracks and 3D.

## ➤ **Nicolas Moës, John Dolbow, and Ted Belytschko (1999)**

- Incorporating a discontinuous field across the crack faces away from the crack tip.
- Independent of remeshing

# Historical Perspective

## ➤ **Stolarska et al. (2001 )**

- Apply level set method within the framework of X-FEM
- The level set method is used to represent the crack location, including the location of crack tips

## ➤ **Sukumar et al. (2001 )**

- Proposed enrichment function for holes and inclusion



# Historical Perspective

## ➤ **Réthoré et al (2005)**

- Discuss the mathematical properties of X-FEM
- Prove the stability of the numerical scheme in the linear case

## ➤ **Chessa and Belytschko (2004-2005 )**

- Extend the enriching shape function in time axis
- space-time extended finite element method

## ➤ **Belytschko (2006)**

- mesh-free method
- no additional unknowns are introduced at the nodes whose supports are crossed by discontinuities

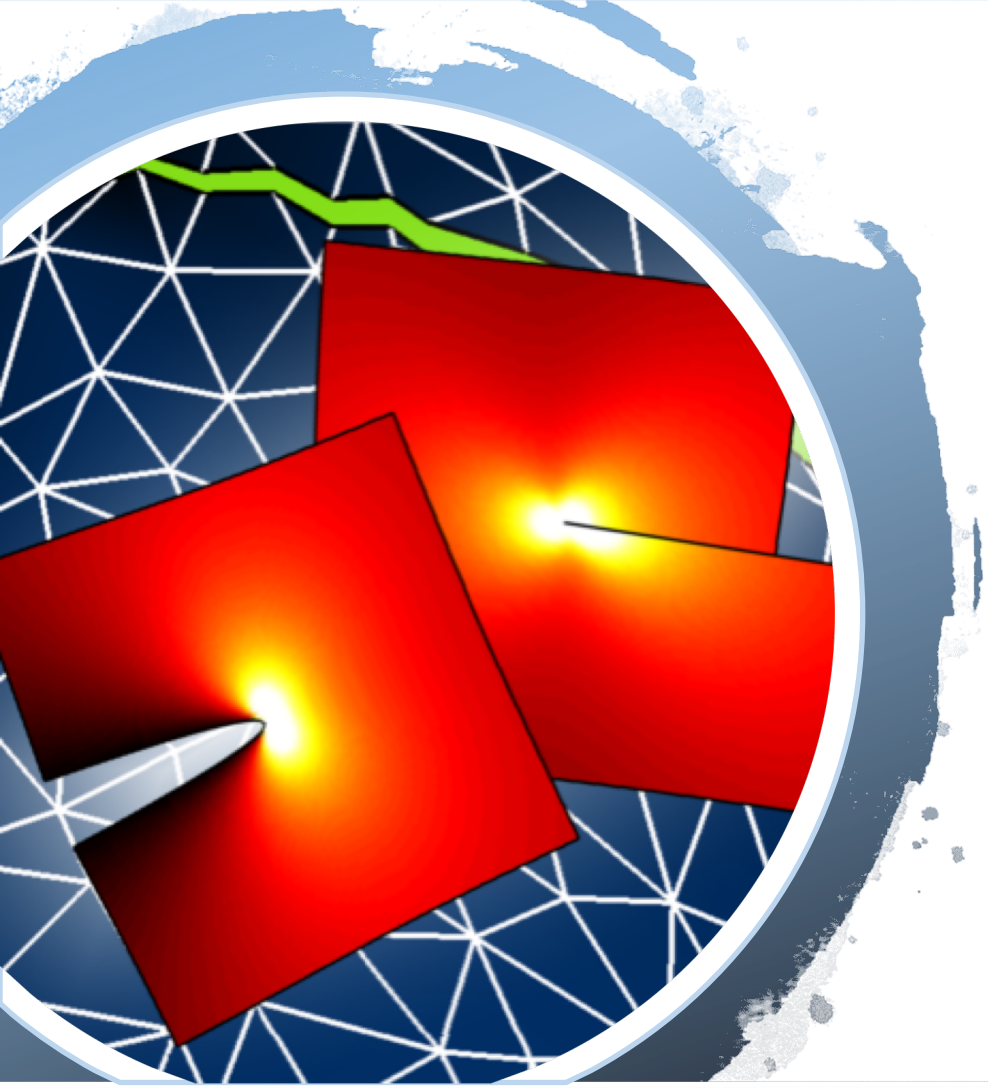
# Historical Perspective

## ➤ Gravouil(2007)

- Model of frictional contact along crack faces via X-FEM

## ➤ Fang and Jin(2007)

- X-FEM algorithm was coupled with commercial software ABAQUS



# General Principles

# Enrichment Function

## ➤ Goal

- Increase the accuracy of the approximation by including information of the analytical solution.

## ➤ Ways of enrichment

- Intrinsic enrichment: Enrich the basic vector
- Extrinsic enrichment: Enrich the approximation

# Enrichment Function

## ➤ Intrinsic Enrichment

- Enhance the approximation space  $u(x)$  by including the new basis functions.

- $u(x) = \sum_{i=1}^p \hat{N}_i(x) \bar{a}_i$        $\hat{N}(x) = \langle N^{std}(x), N^{enr}(x) \rangle$

- $N^{std}(x)$  - Standard polynomial functions  $N_i(x)$
- $N^{enr}(x)$  - Enriched shape functions obtained from  $N_i(x)p_j(x)$
- $\bar{a}$  - A vector of coefficients obtained from one of the least-squares techniques

No additional unknowns

# Enrichment Function

## ➤ Extrinsic Enrichment

- Enrich the approximation by adding the enrichment functions to the standard approximation.
- \*Local\* extrinsic enrichment, instead of enriching whole domain of the solution.
- Shows a systematical error in partially enriched elements.

# Enrichment Function

## ➤ Extrinsic Enrichment

- The general enhanced solution field in the X-FEM:

$$u(\mathbf{x}) = \sum_{i=1}^N N_i(\mathbf{x}) \bar{u}_i + \sum_{k=1}^p \sum_{j=1}^{M_k} \bar{N}_j(\mathbf{x}) \psi_k(\mathbf{x}) \bar{a}_{kj}$$

Additional unknowns

Enrichment

- $M_k \subseteq N$  - sets of nodal points, enriched by functions  $\psi_k(\mathbf{x})$
- $\psi_k(\mathbf{x})$  - Enrichment functions

# Enrichment Function

## ➤ Different techniques used for enrichment function

- Signed distance function
- Level set function
- Heaviside jump function
- ...

## ➤ Dependency on the conditions of problem

- Ex. Discontinuity
  - Different types of material properties → Level Set Function
  - Different displacement fields on either sides of the discontinuity → Heaviside Function



# Enrichment Function

## ➤ Level Set Method

- Definition: A numerical technique for tracking moving interfaces.
- The interface is represented as the zero level set of a function
  - One dimension (time) higher than the dimension of the interface
  - Evolved by solving the hyperbolic conservation laws
  - Independent of element mesh
- Most common function: signed distance function

# Enrichment Function

## ➤ Signed Distance Function

- $\varphi(\mathbf{x}) = \|\mathbf{x} - \mathbf{x}^*\| \text{sign}(\mathbf{n}_{\Gamma_d}(\mathbf{x} - \mathbf{x}^*))$
- $\mathbf{x}^*$  - the closest point projection of  $\mathbf{x}$  onto the discontinuity  $\Gamma_d$
- $\mathbf{n}_{\Gamma_d}$  - the normal vector to the interface at point  $\mathbf{x}^*$
- $\|\mathbf{x} - \mathbf{x}^*\|$  - specifies the distance of point  $\mathbf{x}$  to discontinuity  $\Gamma_d$

# Enrichment Function

## ➤ Heaviside Function

- $H(x) = \begin{cases} 0, & \text{if } \varphi(x) < 0 \\ 1, & \text{if } \varphi(x) > 0 \end{cases}$        $H(x) = \begin{cases} -1, & \text{if } \varphi(x) < 0 \\ +1, & \text{if } \varphi(x) > 0 \end{cases}$

- The approximation field can be written as

Signed distance function

- $\mathbf{u}(x) = \sum_{i=1}^N N_i(x) \bar{\mathbf{u}}_i + \sum_{j=1}^M N_j(x) H(x) \bar{\mathbf{a}}_j$

- Basis of definition of the kinematics of the strong discontinuity (a jump in the displacement field).

# Enriched Shape Function

## ➤ Describing a **Strong** Discontinuity Surface

$$\circ \quad u(x) = \sum_{i=1}^N N_i(x)u_i + \sum_{j=1}^M N_j(x)H(f(x))a_j(t) + \sum_{k=1}^S N_k(x)\phi(x)b_k(t)$$

Crack-crossed

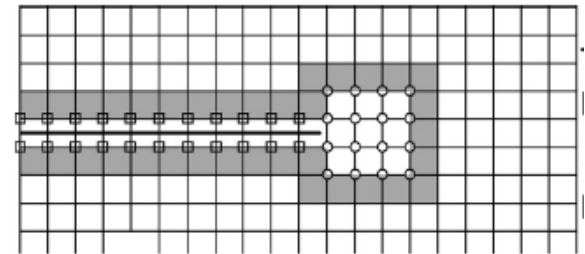
Crack-embedded

- $a_j, b_k$  - Enriched DOFs at the element node

$$\circ \quad H(x) = \begin{cases} -1, & \text{if } x < 0 \\ +1, & \text{if } x \geq 0 \end{cases}$$

$$\circ \quad f(x) = \min\|x - x^*\| \text{sign}(n_{\Gamma_d}(x - x^*))$$

$$\circ \quad \phi(x) = \left[ \sqrt{r} \sin \frac{\theta}{2}, \sqrt{r} \sin \frac{\theta}{2} \sin \theta, \sqrt{r} \cos \frac{\theta}{2}, \sqrt{r} \cos \frac{\theta}{2} \sin \theta \right]$$



# Enriched Shape Function

## ➤ Describing a **Weak** Discontinuity Surface

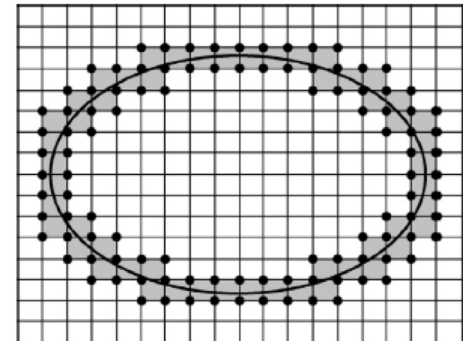
- Displacement field continues at the interface
- Derivative of displacement field (strain field) is discontinuous
  - Material difference

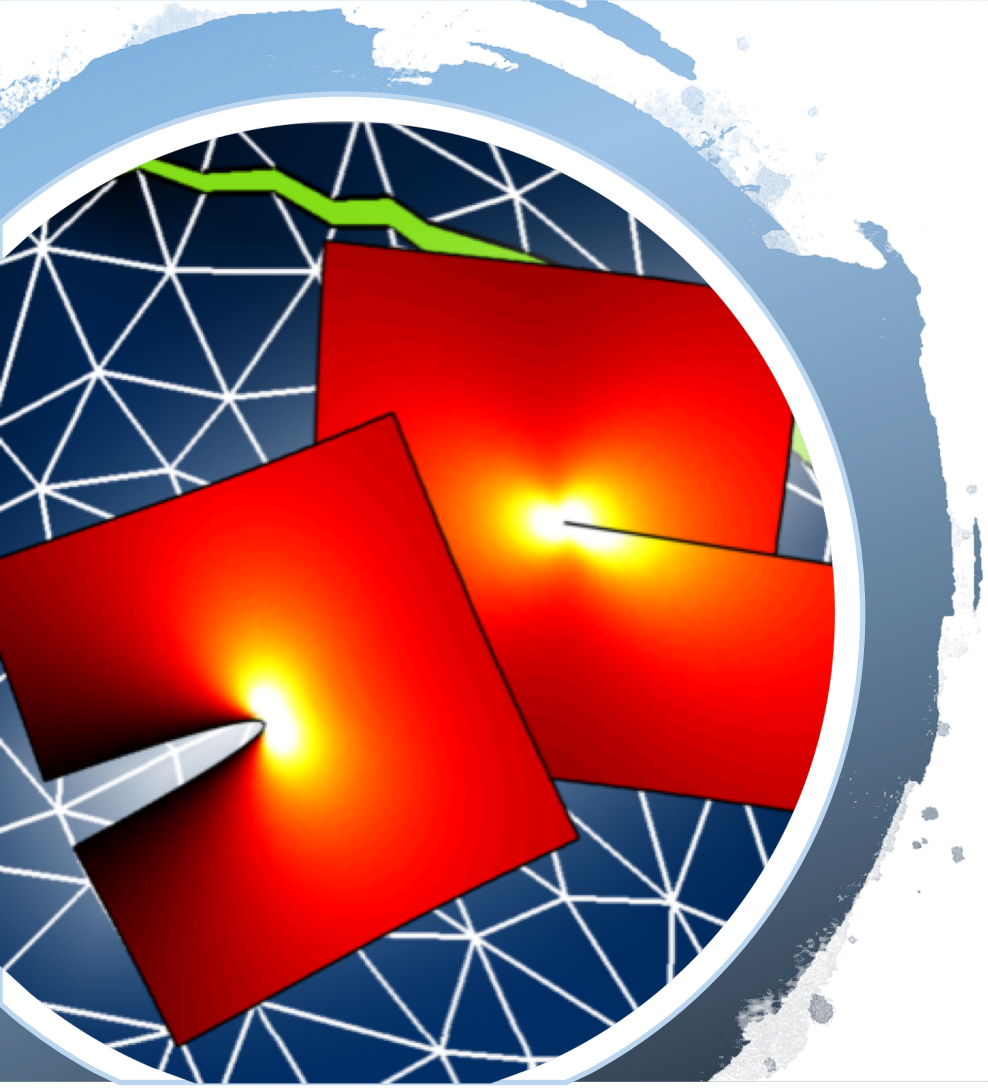
- $$\mathbf{u}(\mathbf{x}, t) = \sum_{i=1}^N N_i(\mathbf{x}) \mathbf{u}_i(t) + \sum_{j=1}^M N_j(\mathbf{x}) \phi(\mathbf{x}) \mathbf{q}_j$$

Interface

- $$\phi(\mathbf{x}) = \sum_i |f(\mathbf{x}_i)| N_i(\mathbf{x}) - |\sum_i |f(\mathbf{x}_i)| N_i(\mathbf{x})|$$

- $\mathbf{q}_j$  - the new added DOF at the node





# Governing Equations

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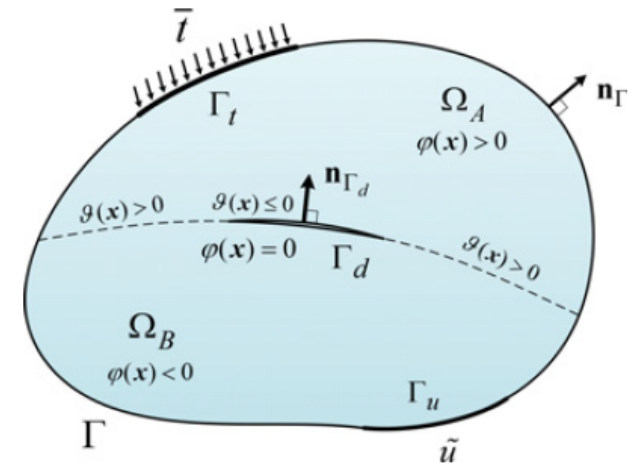
# Governing Equation

➤ **Strong Form of the Equilibrium Equation:**

$$\nabla \cdot \sigma + b = 0 \quad \text{in } \Omega$$

➤ **Boundary Conditions:**

- Displacement (Dirichlet) B.C.:  $u = \tilde{u}$  on  $\Gamma_u$
- Traction (Neumann) B.C.:  $\sigma \cdot n_\Gamma = \tilde{t}$  on  $\Gamma_t$
- Internal B.C.:  $\sigma \cdot n_{\Gamma_d} = \bar{t}_d$  on  $\Gamma_d$



- For domain with strong discontinuity,  $\sigma \cdot n_{\Gamma_d} = 0$

# Governing Equation

## ➤ Weak Form formulation of the Equilibrium Equation:

$\nabla \cdot \boldsymbol{\sigma} + \mathbf{b} = 0$  Multiplying by the test functions,  $\delta \mathbf{u}(x, t)$ , and integrate over the domain  $\Omega$

$$\int_{\Omega} \delta \mathbf{u}(x, t) (\nabla \cdot \boldsymbol{\sigma} + \mathbf{b}) d\Omega = 0$$

↓ Divergence theorem

$$\int_{\Omega} \nabla \delta \mathbf{u} : \boldsymbol{\sigma} d\Omega + \int_{\Gamma_d} [\delta \mathbf{u} \cdot \boldsymbol{\sigma}] \mathbf{n}_{\Gamma_d} d\Gamma - \int_{\Gamma_t} \delta \mathbf{u} \cdot \bar{\mathbf{t}} d\Gamma - \int_{\Omega} \delta \mathbf{u} \cdot \mathbf{b} d\Omega = 0$$

↓

$$\int_{\Omega} \nabla \delta \mathbf{u} : \boldsymbol{\sigma} d\Omega - \int_{\Gamma_t} \delta \mathbf{u} \cdot \bar{\mathbf{t}} d\Gamma - \int_{\Omega} \delta \mathbf{u} \cdot \mathbf{b} d\Omega = 0$$

Weak Discontinuities

$$\int_{\Gamma_d} [\delta \mathbf{u} \cdot \boldsymbol{\sigma}] \mathbf{n}_{\Gamma_d} d\Gamma = \int_{\Gamma_d} [\delta \mathbf{u} \cdot \bar{\mathbf{t}}_d] d\Gamma = \int_{\Gamma_d} (\delta \mathbf{u}^+ \bar{\mathbf{t}}_d^+ - \delta \mathbf{u}^- \bar{\mathbf{t}}_d^-) d\Gamma = 0$$

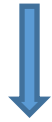
Strong Discontinuities

$$\int_{\Gamma_d} [\delta \mathbf{u} \cdot \boldsymbol{\sigma}] \mathbf{n}_{\Gamma_d} d\Gamma = \int_{\Gamma_d} [\delta \mathbf{u} \cdot \bar{\mathbf{t}}_d] d\Gamma = \int_{\Gamma_d} (\delta \mathbf{u}^+ - \delta \mathbf{u}^-) \bar{\mathbf{t}}_d d\Gamma = 0$$



# Governing Equation

$$\int_{\Omega} \nabla \delta \mathbf{u} : \boldsymbol{\sigma} d\Omega - \int_{\Gamma_t} \delta \mathbf{u} \cdot \bar{\mathbf{t}} d\Gamma - \int_{\Omega} \delta \mathbf{u} \cdot \mathbf{b} d\Omega = 0$$



$$K\bar{\mathbf{U}} - \mathbf{F} = 0$$

- $\mathbf{K}$ , total stiffness matrix
- $\mathbf{F}$ , external force vector
- $\bar{\mathbf{U}}$ , vector of degrees of nodal freedom (classical and enriched)

# Governing Equation

$$K\bar{U} - F = 0 \quad \Rightarrow \quad \begin{bmatrix} \mathbf{K}_{uu} & \mathbf{K}_{ua} \\ \mathbf{K}_{au} & \mathbf{K}_{aa} \end{bmatrix} \begin{Bmatrix} \bar{\mathbf{u}} \\ \bar{\mathbf{a}} \end{Bmatrix} = \begin{Bmatrix} \mathbf{F}_u \\ \mathbf{F}_a \end{Bmatrix}$$

$$\mathbf{K} = \begin{bmatrix} \int_{\Omega} (\mathbf{B}^{std})^T \mathbf{D} \mathbf{B}^{std} d\Omega & \int_{\Omega} (\mathbf{B}^{std})^T \mathbf{D} \mathbf{B}^{enr} d\Omega \\ \int_{\Omega} (\mathbf{B}^{enr})^T \mathbf{D} \mathbf{B}^{std} d\Omega & \int_{\Omega} (\mathbf{B}^{enr})^T \mathbf{D} \mathbf{B}^{enr} d\Omega \end{bmatrix}$$

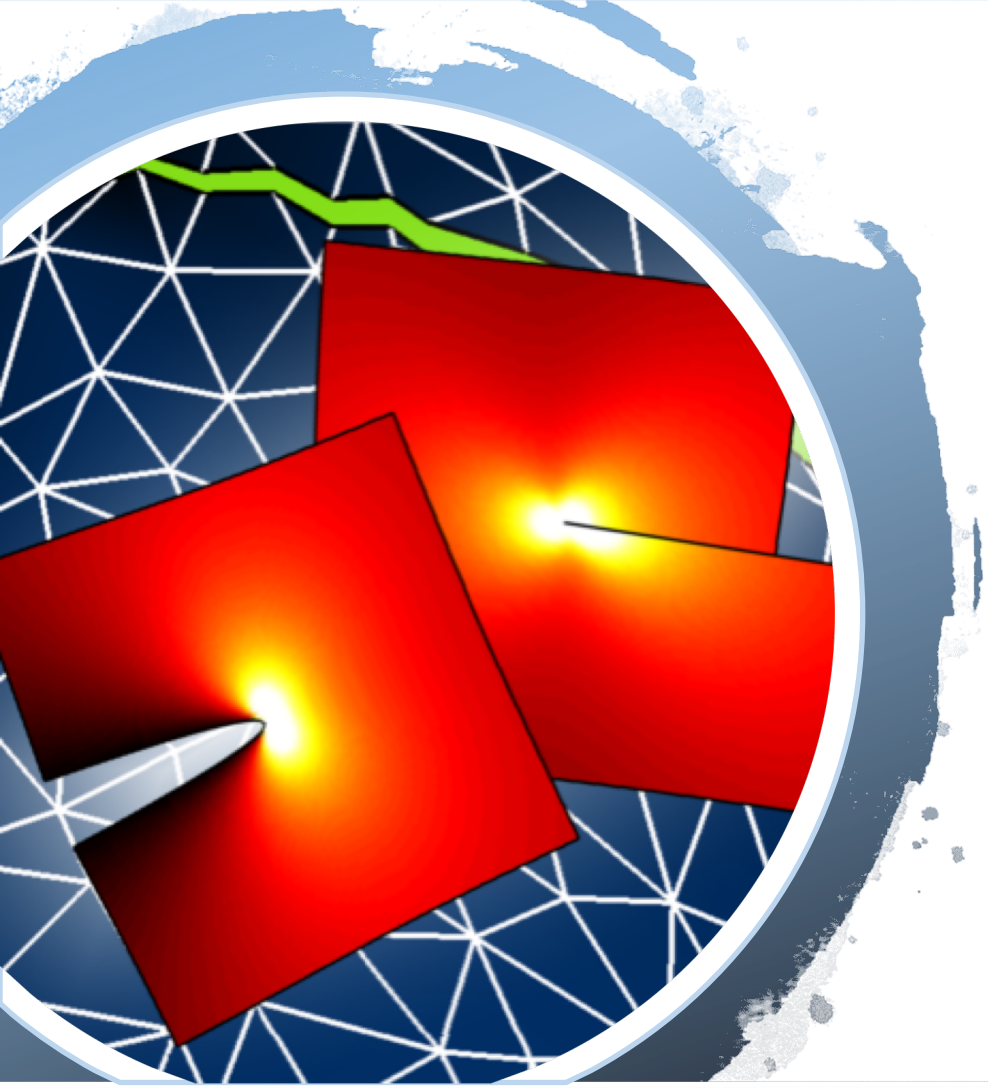
$$\mathbf{B}_i^{std} = \begin{bmatrix} \partial N_i / \partial x & 0 \\ 0 & \partial N_i / \partial y \\ \partial N_i / \partial y & \partial N_i / \partial x \end{bmatrix}$$

$$\mathbf{B}_j^{enr} = \begin{bmatrix} \partial [N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j))] / \partial x & 0 \\ 0 & \partial [N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j))] / \partial y \\ \partial [N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j))] / \partial y & \partial [N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j))] / \partial x \end{bmatrix}$$

$$\mathbf{F} = \begin{Bmatrix} \int_{\Gamma_i} (\mathbf{N}^{std})^T \bar{\mathbf{t}} d\Gamma + \int_{\Omega} (\mathbf{N}^{std})^T \mathbf{b} d\Omega \\ \int_{\Gamma_i} (\mathbf{N}^{enr})^T \bar{\mathbf{t}} d\Gamma + \int_{\Omega} (\mathbf{N}^{enr})^T \mathbf{b} d\Omega \end{Bmatrix}$$

$$\mathbf{N}_i^{std} = \begin{bmatrix} N_i(\mathbf{x}) & 0 \\ 0 & N_i(\mathbf{x}) \end{bmatrix}$$

$$\mathbf{N}_j^{enr} = \begin{bmatrix} N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j)) & 0 \\ 0 & N_j(\mathbf{x})(\psi(\mathbf{x}) - \psi(\mathbf{x}_j)) \end{bmatrix}$$



# Hand-Calculation Examples

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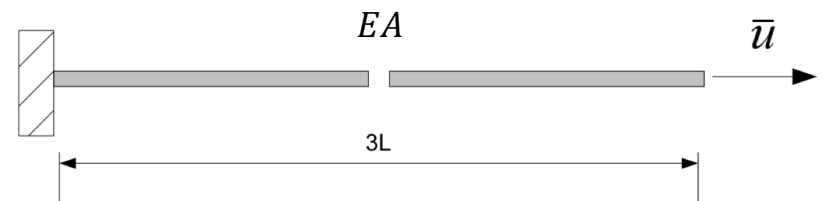
# Hand-Calculation

Consider a 1D bar of length  $3L$ .

Let  $E$  be the elastic moduli and  $A$  be the cross-sectional area of the bar.

The bar is subjected to a prescribed displacement at the end while the other end of the bar is fixed.

The bar is cracked at its mid length,  $L=1.5L$ .

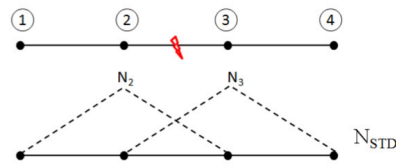


# Hand-Calculation

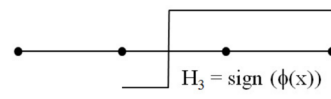
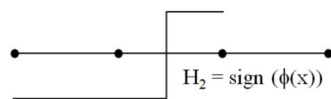
## XFEM Solution with Non-aligned Mesh

$$u_{XFEM} = \sum_{i=1}^N N_i u_i + \sum_{j=1}^M N_j H(x) a_j$$

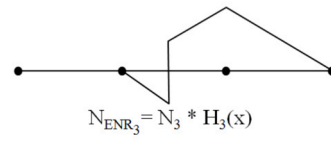
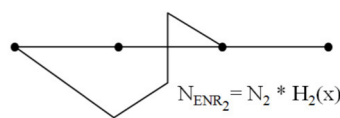
Enriched basis function for a strong discontinuity in 1D



$$[N] = \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right]$$



$$H(x) = \begin{cases} +1 & x - x_0 > 0 \\ 0 & x = x_0 \\ -1 & x - x_0 < 0 \end{cases}$$

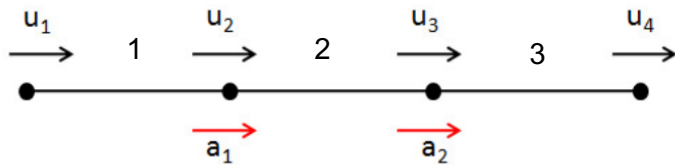


$$u(x) = \sum_{i=1}^N N_i(x) u_i + \sum_{j=1}^M N_j(x) H(f(x)) a_j(t) + \sum_{k=1}^S N_k(x) \phi(x) b_k(t)$$

# Hand-Calculation

## ➤ XFEM Solution with Non-aligned Mesh

Element No.1



The step function  $H(x) = -1$

$$N_{std}^u = \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right] \quad N_{enr}^a = H \left[ \frac{x}{L} \right] = \left[ -\frac{x}{L} \right]$$

$$B_{std}^u = \left[ -\frac{1}{L} \quad \frac{1}{L} \right] \quad B_{enr}^a = H \left[ \frac{1}{L} \right] = \left[ -\frac{1}{L} \right]$$

$$K_{uu} = EA \int_0^L (B_{std}^u)^T B_{std}^u dx = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K_{ua} = EA \int_0^L (B_{std}^u)^T B_{enr}^a dx = \frac{EA}{L} \begin{bmatrix} -1 \\ 1 \end{bmatrix}$$

$$K_{au} = K_{ua}^T$$

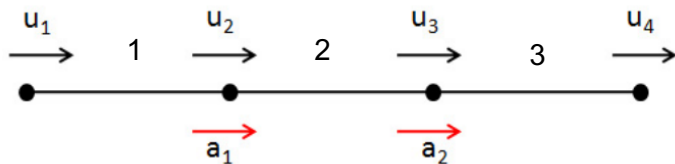
$$K_{aa} = EA \int_0^L (B_{enr}^a)^T B_{enr}^a dx = \frac{EA}{L}$$

$$K_{e^1} = \frac{EA}{L} \begin{bmatrix} 1 & -1 & -1 \\ -1 & 1 & 1 \\ -1 & 1 & 1 \end{bmatrix}$$

# Hand-Calculation

## ➤ XFEM Solution with Non-aligned Mesh

Element No.3



The step function  $H(x) = +1$

$$N_{std}^u = \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right] \quad N_{enr}^a = H \left[ 1 - \frac{x}{L} \right] = \left[ 1 - \frac{x}{L} \right]$$

$$B_{std}^u = \left[ -\frac{1}{L} \quad \frac{1}{L} \right] \quad B_{enr}^a = H \left[ -\frac{1}{L} \right] = \left[ -\frac{1}{L} \right]$$

$$K_{uu} = EA \int_0^L (B_{std}^u)^T B_{std}^u dx = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K_{ua} = EA \int_0^L (B_{std}^u)^T B_{enr}^a dx = \frac{EA}{L} \begin{bmatrix} -1 \\ 1 \end{bmatrix}$$

$$K_{au} = K_{ua}^T$$

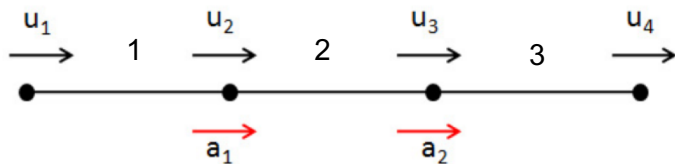
$$K_{aa} = EA \int_0^L (B_{enr}^a)^T B_{enr}^a dx = \frac{EA}{L}$$

$$K_{e^3} = \frac{EA}{L} \begin{bmatrix} 1 & -1 & -1 \\ -1 & 1 & 1 \\ -1 & 1 & 1 \end{bmatrix}$$

# Hand-Calculation

## XFEM Solution with Non-aligned Mesh

Element No.2



The enrichment function  $H(x) = \begin{cases} +1 & x \rightarrow x_0^+ \\ -1 & x \rightarrow x_0^- \end{cases}$

$$N_{std}^u = \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right]$$

$$N_{enr}^a = H \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right] = \begin{cases} \left[ 1 - \frac{x}{L} \quad \frac{x}{L} \right] & x \rightarrow x_0^+ \\ \left[ \frac{x}{L} \quad 1 - \frac{x}{L} \right] & x \rightarrow x_0^- \end{cases}$$

$$B_{std}^u = \left[ -\frac{1}{L} \quad \frac{1}{L} \right]$$

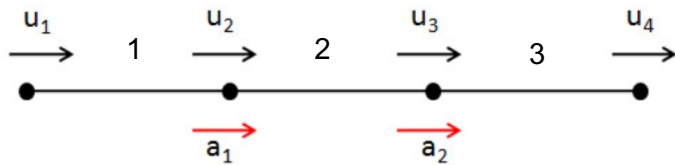
$$B_{enr}^a = H \left[ -\frac{1}{L} \quad \frac{1}{L} \right] = \begin{cases} \left[ -\frac{1}{L} \quad \frac{1}{L} \right] & x \rightarrow x_0^+ \\ \left[ \frac{1}{L} \quad -\frac{1}{L} \right] & x \rightarrow x_0^- \end{cases}$$



# Hand-Calculation

## XFEM Solution with Non-aligned Mesh

Element No.2



$$K_{uu} = EA \int_0^L (B_{std}^u)^T B_{std}^u dx = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K_{ua} = K_{ua}^+ + K_{ua}^-$$

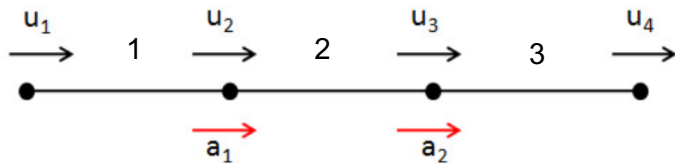
$$K_{au} = K_{au}^+ + K_{au}^-$$

$$K_{aa} = K_{aa}^+ + K_{aa}^-$$

# Hand-Calculation

## XFEM Solution with Non-aligned Mesh

Element No.2



Integrating on +, the enrichment function  $H(x) = +1$

$$K_{ua}^+ = EA \int_0^{\frac{L}{2}} (B_{std}^u)^T B_{enr}^a dx = \frac{EA}{2L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K_{aa}^+ = EA \int_0^{\frac{L}{2}} (B_{enr}^a)^T B_{enr}^a dx = \frac{EA}{2L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

Integrating on -, the enrichment function  $H(x) = -1$

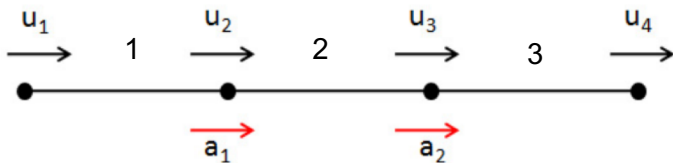
$$K_{ua}^- = EA \int_{\frac{L}{2}}^L (B_{std}^u)^T B_{enr}^a dx = \frac{EA}{2L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$K_{aa}^- = EA \int_{\frac{L}{2}}^L (B_{enr}^a)^T B_{enr}^a dx = \frac{EA}{2L} \begin{bmatrix} -1 & 1 \\ 1 & -1 \end{bmatrix}$$

# Hand-Calculation

## XFEM Solution with Non-aligned Mesh

Element No.2



$$K_{ua} = K_{ua}^+ + K_{ua}^- = \frac{EA}{2L} \begin{bmatrix} 0 & 0 \\ 0 & 0 \end{bmatrix}$$

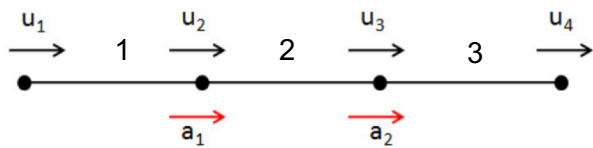
$$K_{aa} = K_{aa}^+ + K_{aa}^- = \frac{EA}{L} \begin{bmatrix} -1 & 1 \\ 1 & -1 \end{bmatrix}$$

$$K_{au} = K_{ua}^T$$

$$K_{e^2} = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 1 & 0 & 0 \\ 0 & 0 & 1 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix}$$

# Hand-Calculation

## XFEM Solution with Non-aligned Mesh



Not a nodal interpolant.

$$K = \frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 & -1 & 0 \\ -1 & 2 & -1 & 0 & 1 & 0 \\ 0 & -1 & 2 & -1 & 0 & -1 \\ 0 & 0 & -1 & 1 & 0 & 1 \\ -1 & 1 & 0 & 0 & 2 & -1 \\ 0 & 0 & -1 & 1 & -1 & 2 \end{bmatrix}$$

$$K\bar{u} - F = 0$$

$$\begin{aligned} u_1 &= 0 \\ u_4 &= \bar{u} \end{aligned}$$

$$\begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \\ a_1 \\ a_2 \end{bmatrix} = \begin{bmatrix} 0 \\ \frac{\bar{u}}{2} \\ \frac{\bar{u}}{2} \\ \bar{u} \\ -\frac{\bar{u}}{2} \\ -\frac{\bar{u}}{2} \end{bmatrix}$$

$$u(X) = N_i u_i + H N_j a_j$$

$$u(x_1) = u_1 = 0$$

$$u(x_2) = u_2 + H(x_2)a_1 = \frac{\bar{u}}{2} - \frac{\bar{u}}{2} = 0$$

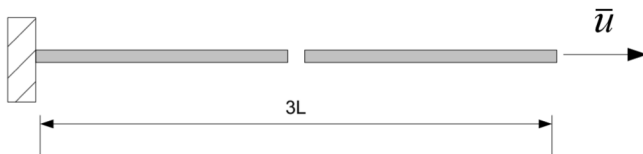
$$u(x_3) = u_3 + H(x_3)a_2 = \frac{\bar{u}}{2} + (-1) \left( -\frac{\bar{u}}{2} \right) = \bar{u}$$

$$u(x_4) = u_4 = \bar{u}$$

Enriched displacement approximation

# Hand-Calculation

## Standard Finite Element Method



The finite element mesh has to be aligned with the crack.



$$K\bar{u} - F = 0$$



$$\frac{EA}{L} \begin{bmatrix} 1 & -1 & 0 & 0 \\ -1 & 2 & -1 & 0 \\ 0 & -1 & 2 & -1 \\ 0 & 0 & -1 & 1 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \\ u_4 \end{bmatrix} = \begin{bmatrix} f_1 \\ f_2 \\ f_3 \\ f_4 \end{bmatrix}$$



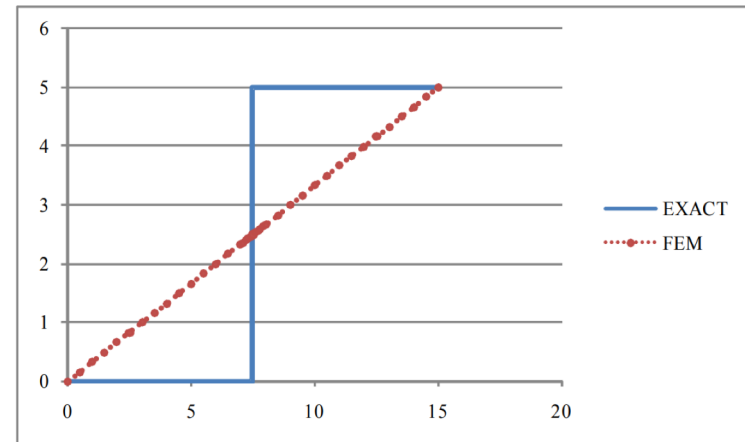
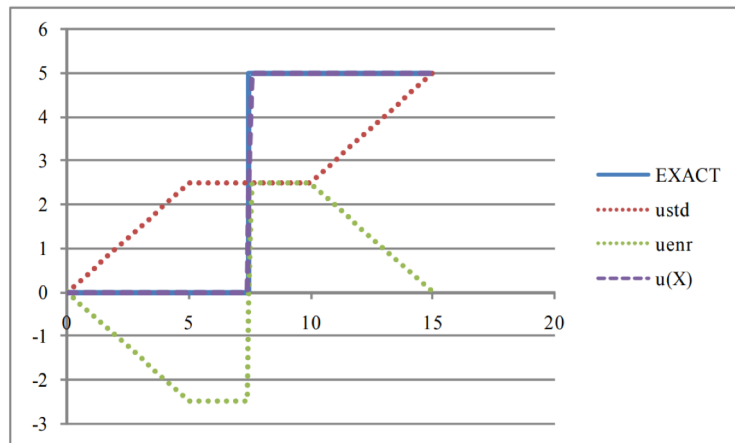
$$u_1 = 0 \quad u_4 = \bar{u}$$

$$u_1 = 0, \quad u_2 = \frac{\bar{u}}{3}, \quad u_3 = \frac{2\bar{u}}{3}, \quad u_4 = \bar{u}$$

# Hand-Calculation

## Compare XFEM Solution with Standard Finite Element Method

Numerical solution of displacement field using XFEM and FEM



# Hand-Calculation

$$u^h(x) = \sum_{i \in I} u_i N_i(x) + \sum_{i \in L} a_i N_i(x) H(x) + \sum_{i \in K_1} N_i(x) \left( \sum_{l=1}^4 b_{i,l}^l F_1^l(x) \right) + \sum_{i \in K_2} N_i(x) \left( \sum_{l=1}^4 b_{i,l}^l F_2^l(x) \right)$$

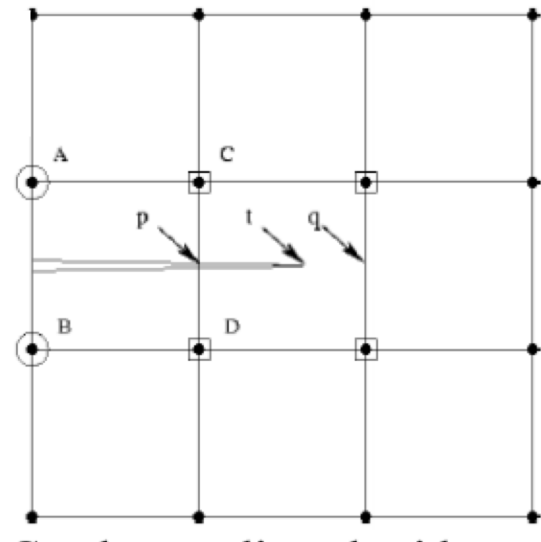
- $I$  is the set of nodes in the mesh. •  $u_i$  is the classical (vectorial) degree of freedom at node  $i$ .
- $N_i$  is the scalar shape function associated to node  $i$ .
- $L \subset I$  is the subset of nodes enriched by the Heaviside function. The corresponding (vectorial) DOF are denoted  $a_i$ .
- $K_1 \subset I$  and  $K_2 \subset I$  are the set of nodes to enrich to model crack tips numbered 1 and 2, respectively. The corresponding degrees of freedom are  $b_{i,l}^1$  and  $b_{i,l}^2, l=1, \dots, 4$ .
- Functions  $F_1^l(x), l=1, \dots, 4$  modeling the crack tip are given in elasticity by :

$$\{F_1^l(x)\} = \left\{ \sqrt{r} \sin\left(\frac{\theta}{2}\right), \sqrt{r} \cos\left(\frac{\theta}{2}\right), \sqrt{r} \sin\left(\frac{\theta}{2}\right) \sin(\theta), \sqrt{r} \cos\left(\frac{\theta}{2}\right) \sin(\theta) \right\}$$

Vector Enrichment:

Scalar Enrichment:

$$u = \sum u_i N_i + \sum K_1 N_i G_1(r, \theta) + \sum K_2 N_i G_2(r, \theta) \quad \bar{u} = \sum u_i N_i + \sum a_i N_i F_1(r, \theta) + \sum b_i N_i F_2(r, \theta) + \sum c_i N_i F_3(r, \theta) + \sum d_i N_i F_4(r, \theta)$$



Source: Dibakar Datta

# Hand-Calculation

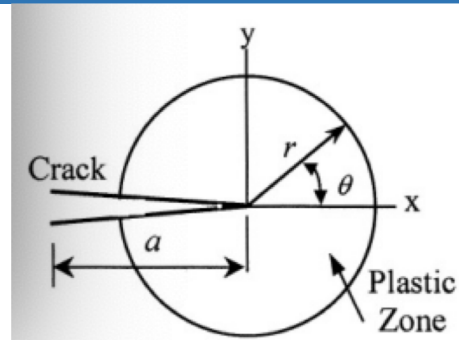


Fig 2.11: Crack tip circular region

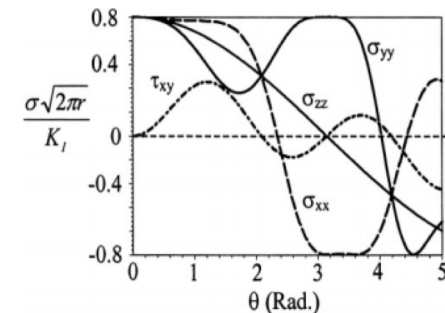


Fig 2.9: Normalized Stress Distribution for Mode I.

Solution for Stress Field:

$$\begin{bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \tau_{xy} \end{bmatrix} = \frac{K_I}{\sqrt{2\pi r}} \cos \frac{\theta}{2} \begin{bmatrix} 1 - \sin \frac{\theta}{2} \sin \frac{3\theta}{2} \\ 1 + \sin \frac{\theta}{2} \sin \frac{3\theta}{2} \\ \sin \frac{\theta}{2} \cos \frac{3\theta}{2} \end{bmatrix}$$

Solution for Displacement Field:

$$\begin{bmatrix} \mu_x \\ \mu_y \\ \mu_z \end{bmatrix} = \frac{2K_I}{\sqrt{2\pi E}} \begin{bmatrix} \sqrt{r} \cos \frac{\theta}{2} [(1-\nu) + (1+\nu) \sin^2 \frac{\theta}{2}] \\ \sqrt{r} \sin \frac{\theta}{2} [2 - (1+\nu)] \cos^2 \frac{\theta}{2} \\ -\frac{\nu B}{\sqrt{r}} \cos \frac{\theta}{2} \end{bmatrix}$$

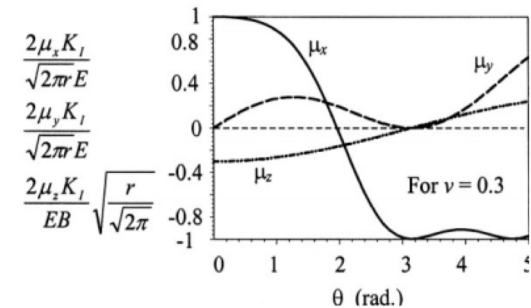


Fig 2.10: Normalized Displacement Distribution for Mode I.

Source: Dibakar Datta



# Hand-Calculation

Fig 2.11: Comparison of error for different scalar type of enrichment radius for polynomial degree 1

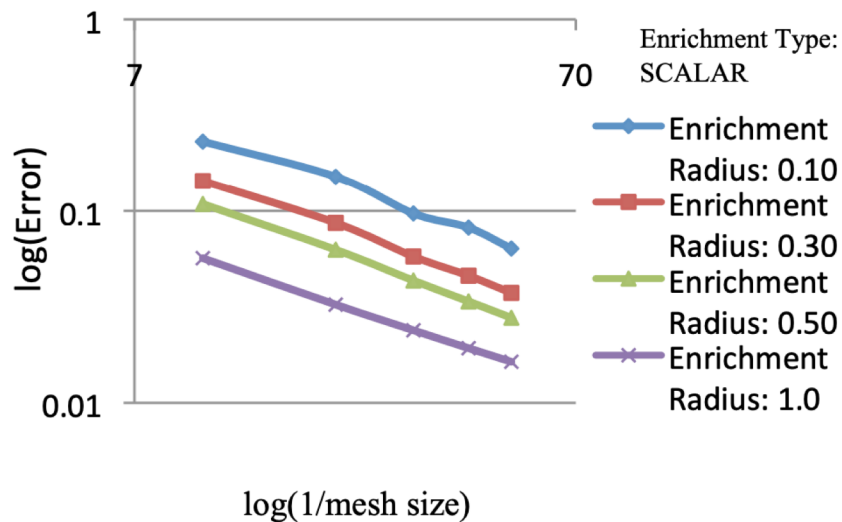
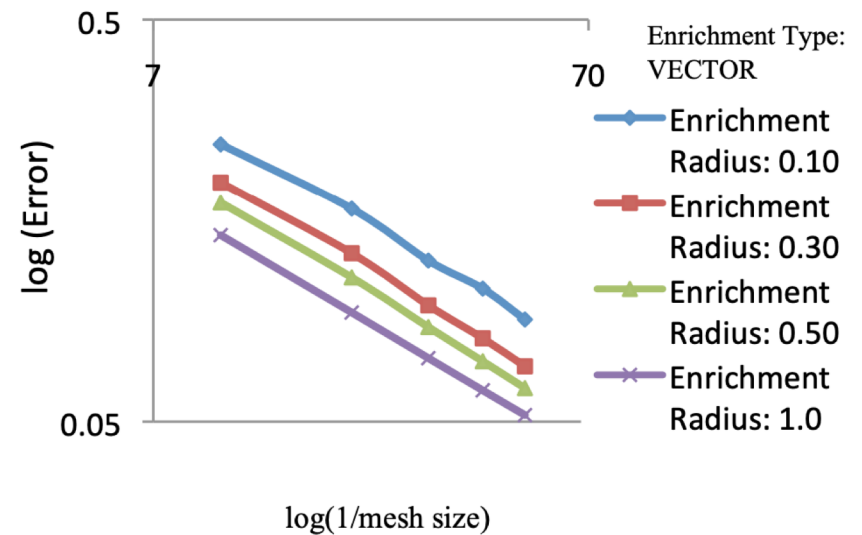
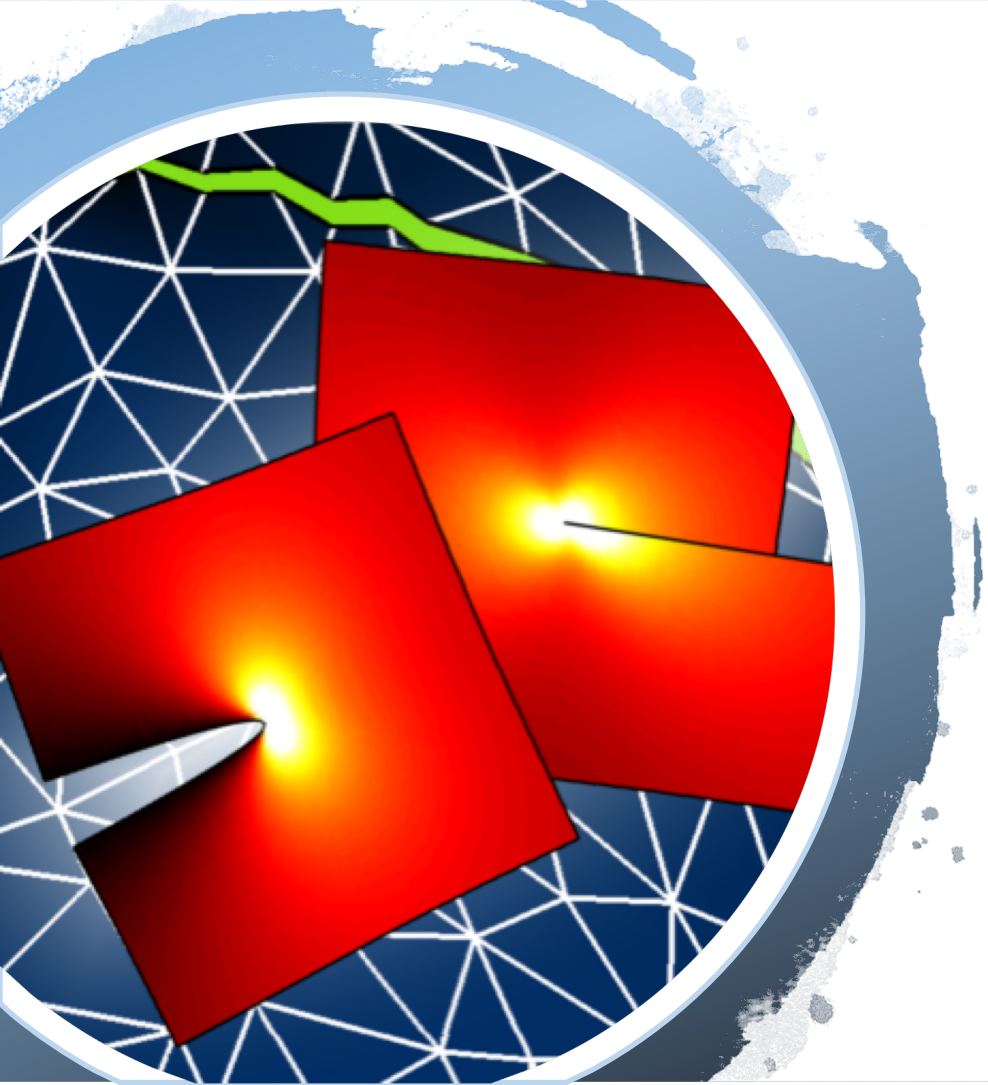


Fig 2.12: Comparison of error for different vector type of enrichment radius for polynomial degree 1

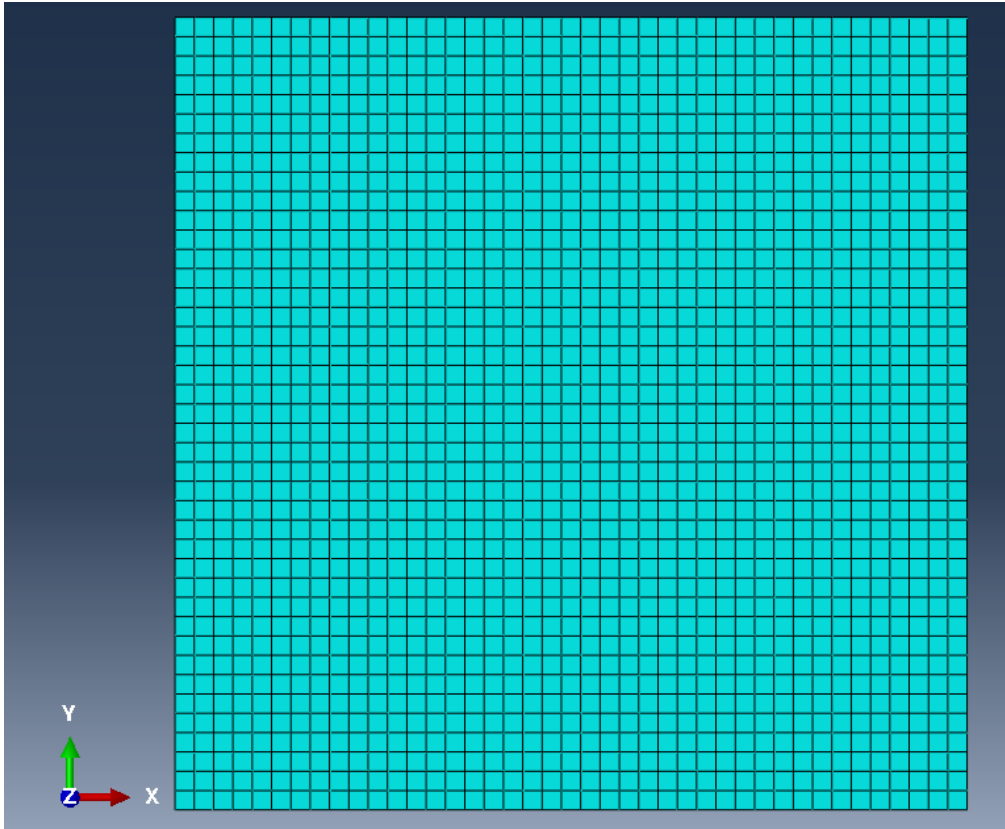


Source: Dibakar Datta



# Numerical Example

# Numerical Example

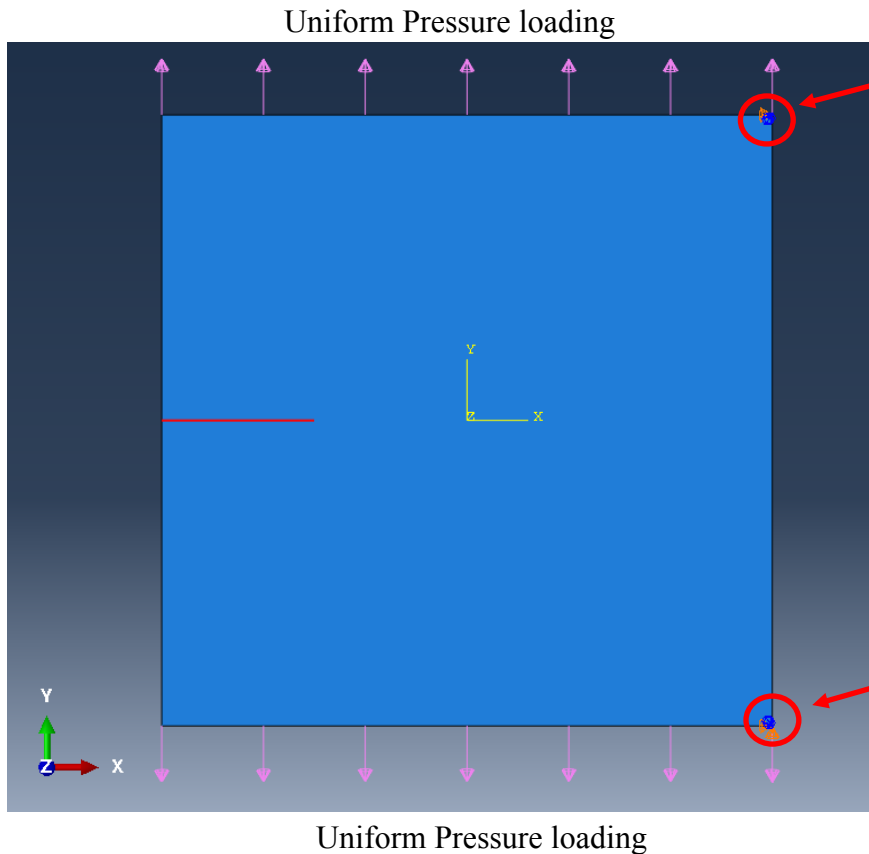


## 2D Static Edge Crack

Un-cracked Domain:

- 2D Planar,
- $\begin{bmatrix} (-2,2) & (2,2) \\ (-2,-2) & (2,-2) \end{bmatrix}$
- 41x41
- Aluminum
- $E = 70 \text{ Gpa}$
- $\nu = 0.33$
- Max. Principle Stress: 500 Mpa

# Numerical Example



Roller B.C.:  
 $u_1 = u_3 = 0$

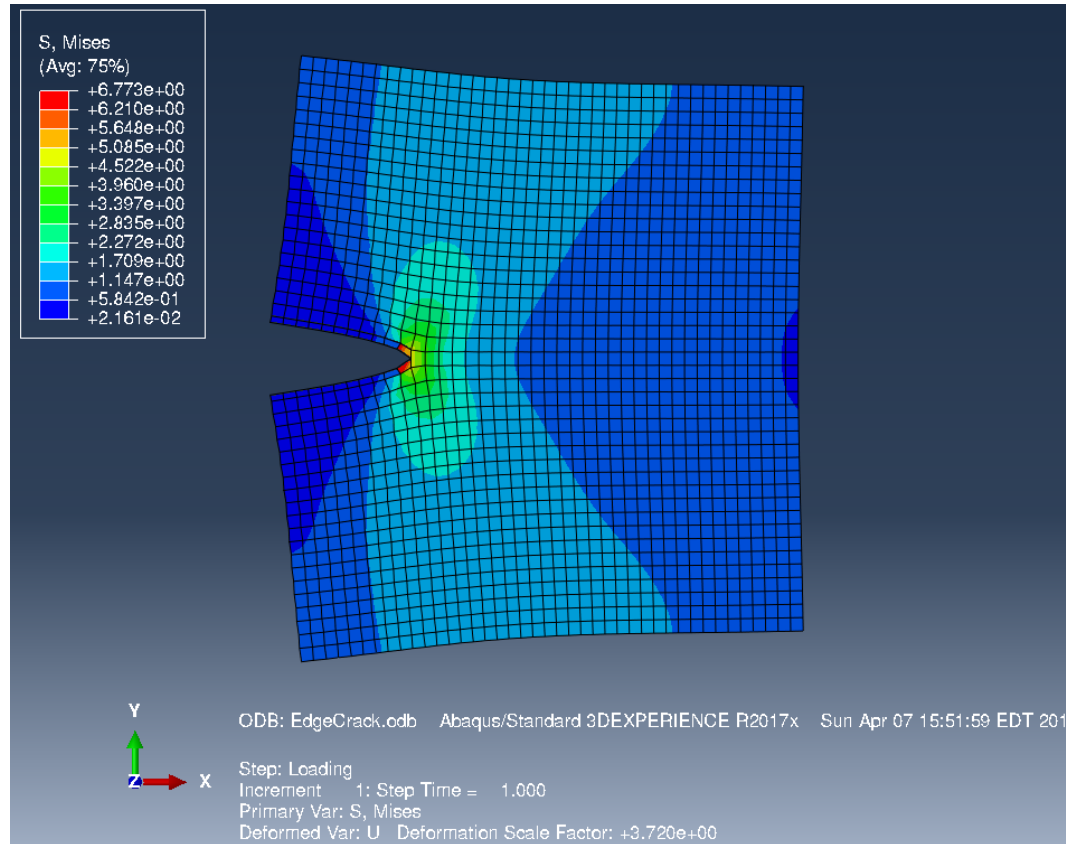
## 2D Static Edge Crack

Cracked Domain:

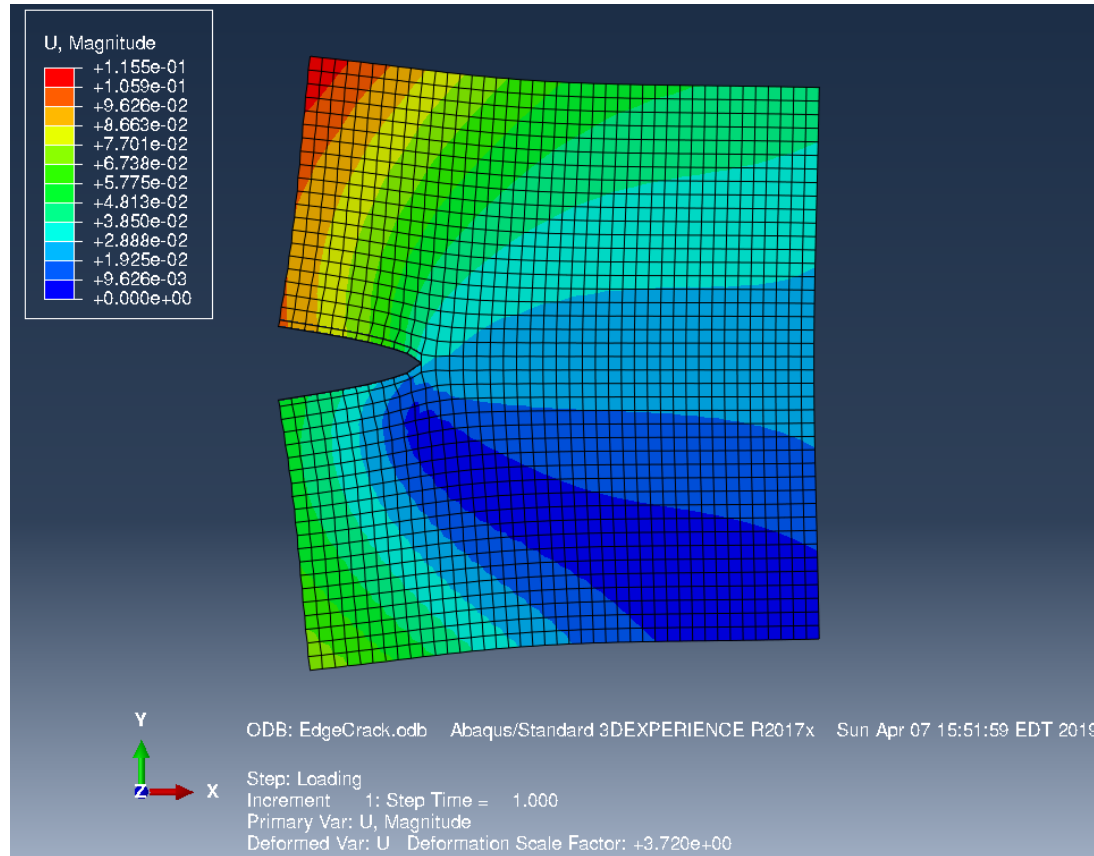
- 2D Planar with wire
- $[(-2,0) \quad (-1,0)]$  for the crack

Fixed B.C.:  
 $u_1 = u_2 = u_3 = 0$

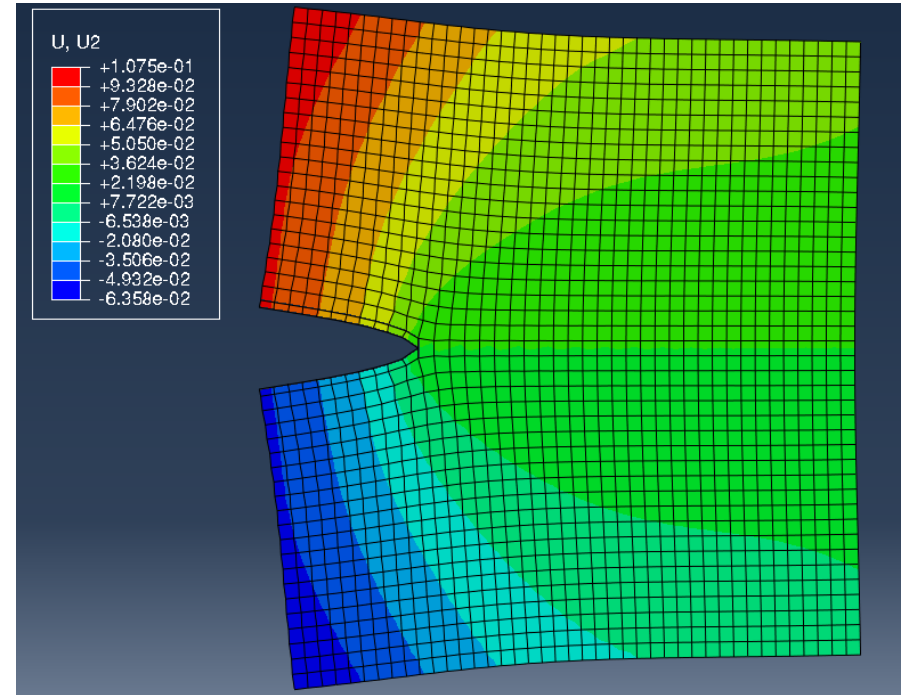
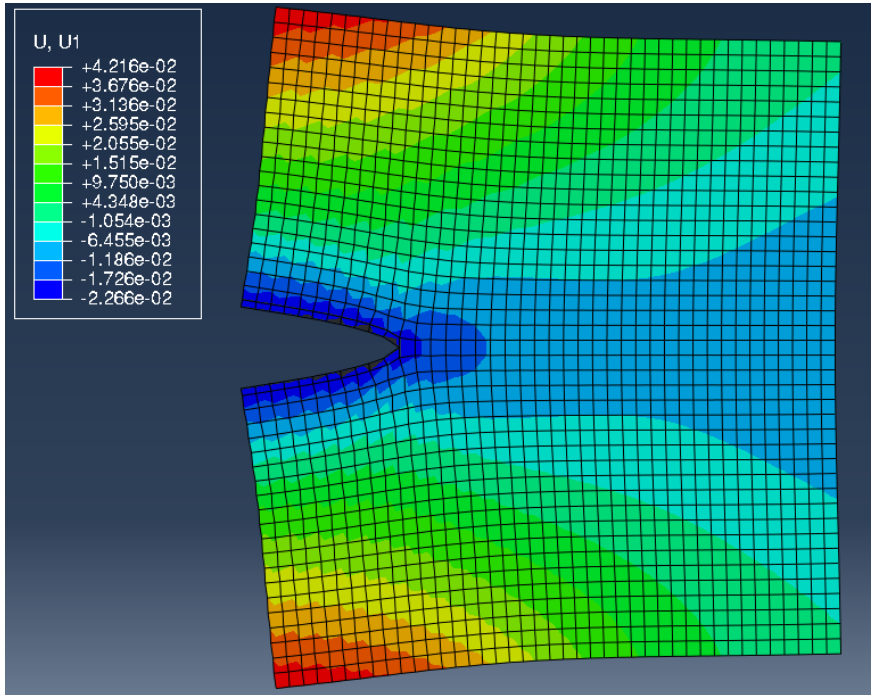
# Numerical Example



# Numerical Example

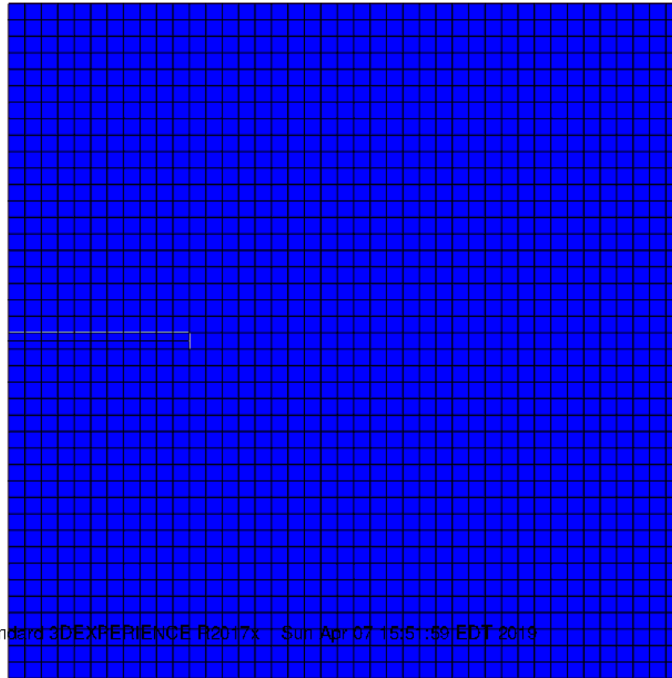
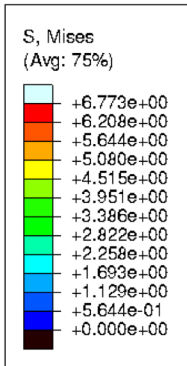


# Numerical Example



# Numerical Example

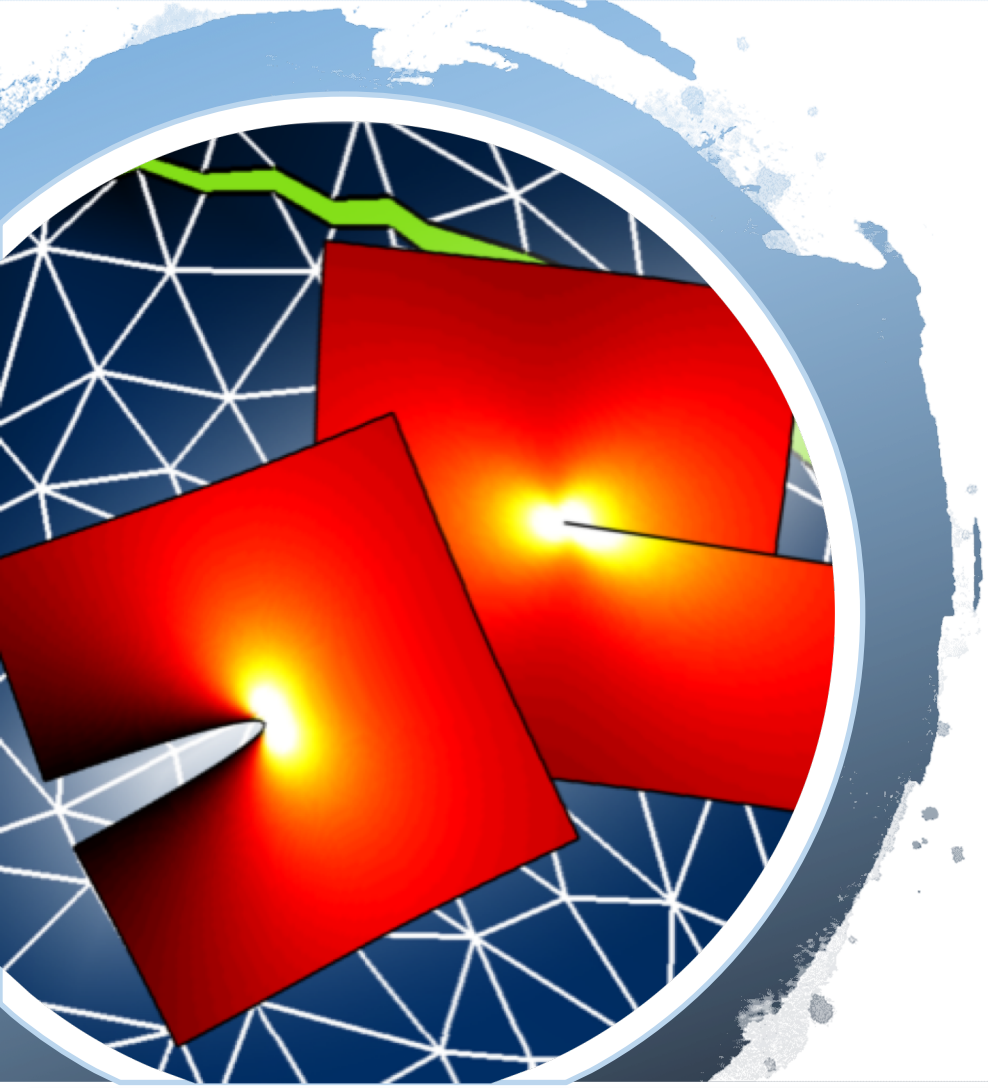
Scale Factor: +0.00



ODB: EdgeCrack.odb Abaqus/Standard 3DEXPERIENCE R2017x Sun Apr 07 15:51:59 EDT 2019

Step: Loading  
Increment 1: Step Time = 1.000  
Primary Var: S, Mises  
Deformed Var: U Deformation Scale Factor: +3.720e+00



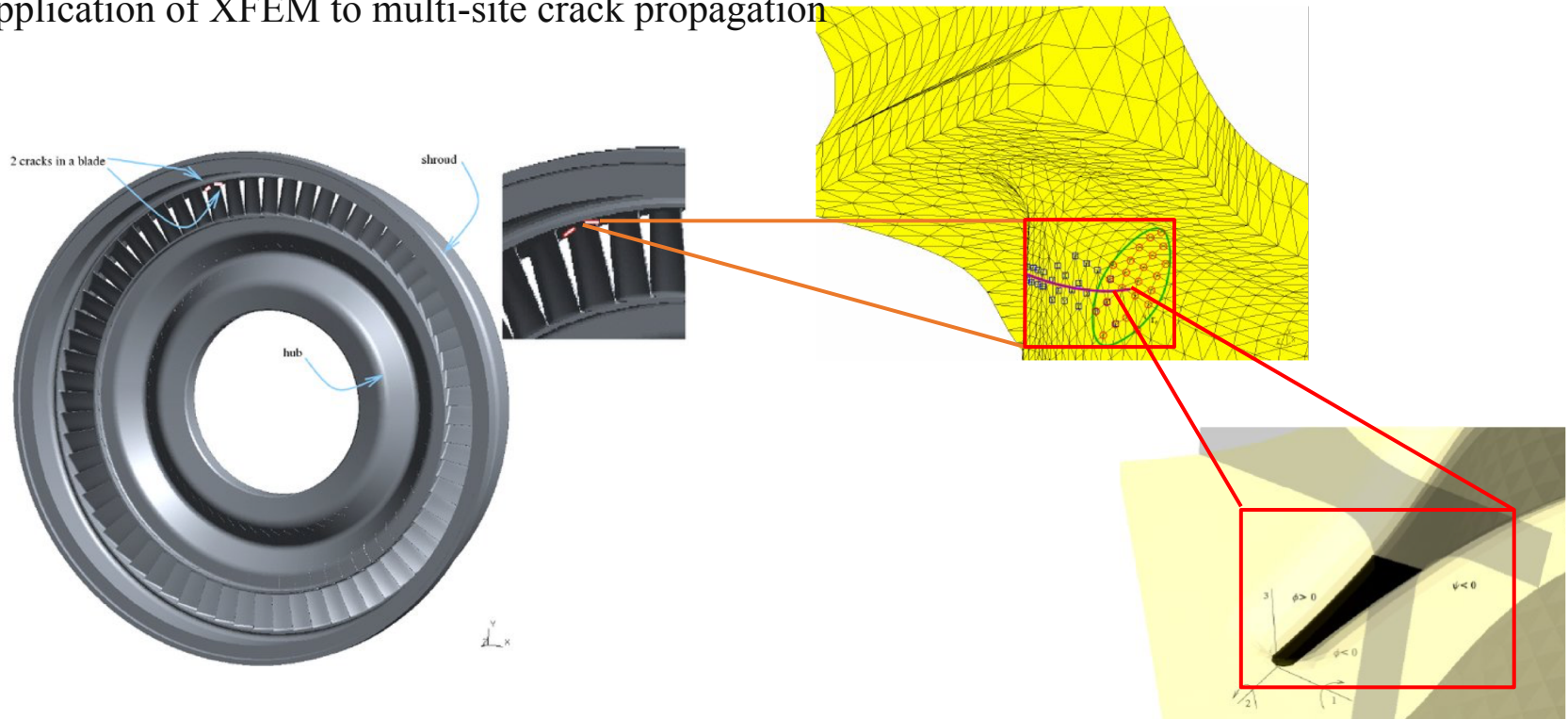


# Example Applications

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# Example Applications

Application of XFEM to multi-site crack propagation

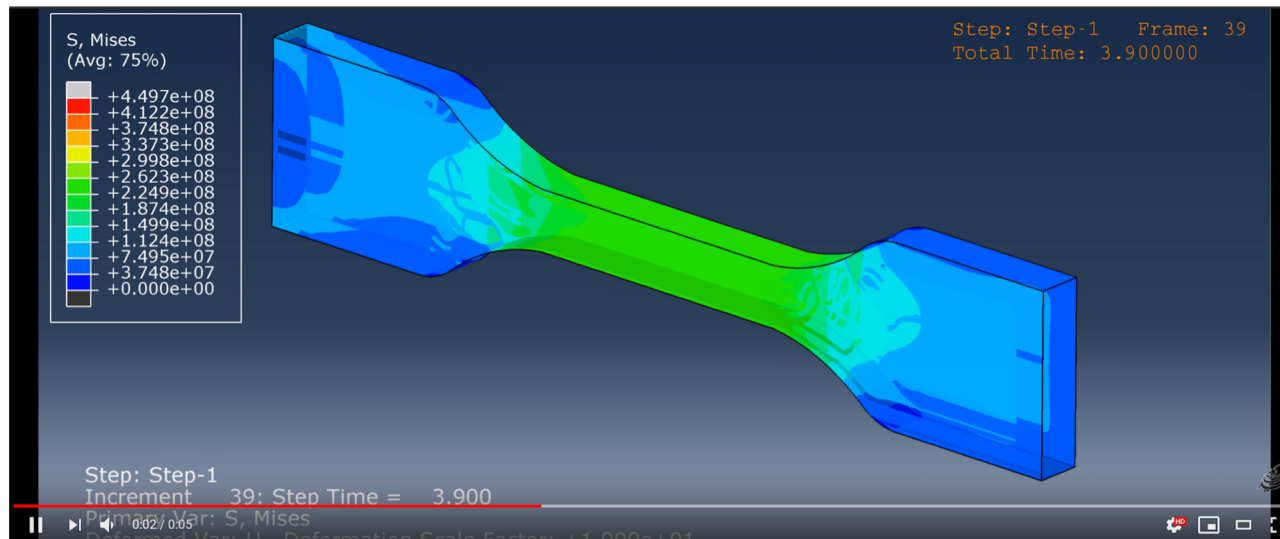


Duflot, Marc, et al. "Application of XFEM to multi-site crack propagation." *Engng Fract Mech*, submitted for publication(2008).

# Example Applications

Abaqus XFEM simulation for tensile test

<https://www.youtube.com/watch?v=QJws0SaGdII>



**Thank you!**